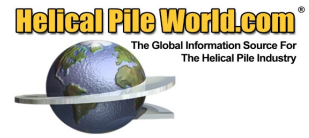


ECP Helical Piles, Tiebacks & Anchors		Ultimate Capacity Based Upon Torque (kips - kN) (1) (2)	Helix Bearing Plate Grade & Thickness (in - mm)	Section Coupling Method	Building Code Certifications
Round Corner Square Bar (RCS)					
Model TAF-150	1.50 in - 38.1 mm C1530, Yield Strength = 90 ksi (min)	Comp = 70 kips - 311 kN Ten = 70 kips - 311 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm std 0.50 in - 12.7 mm opt	(1) 0.75 in - 19.1 mm Grd 8 Bolt	ESR 3559
Model TAF-175	1.75 in - 44.5 mm C1530, Yield Strength = 90 ksi (min)	Comp = 100 kips - 445 kN Ten = 100 kips - 445 kN	ASTM A572 Grade 50 std 0.375 in - 9.5 mm std 0.50 in - 12.7 mm opt	(1) 0.75 in - 19.1 mm Grd 8 Bolt	ESR 3559
Model TAF-200	2.00 in - 50.8 mm C1530, Yield Strength = 90 ksi (min)	Comp = 150 kips - 667 kN Ten = 150 kips - 667 kN	ASTM A572 Grade 80 std .50 in - 12.7 mm std	(1) 0.75 in - 19.1 mm Grd 8 Bolt	none
Model TAF-225	2.25 in - 57.2 mm C1530, Yield Strength = 90 ksi (min)	Comp = 200 kips - 890 kN Ten = 200 kips - 890 kN	ASTM A572 Grade 80 std 0.50 in - 12.7 mm std	(2) 0.75 in - 19.1 mm Grd 8 Bolts	none
Square Tube					
Model HTAF-30S	3.00 in - 76.2 mm 0.3125 in - 7.94 mm Wall	Comp = 150 kips - 1068 kN Ten = 150 kips - 1068 kN	ASTM A572 Grade 80 std 0.50 in - 12.7 mm std	(1) 0.875 in - 22.2 mm Grade 8 Bolt	none
Round Shaft					
Model TAF-288L	O.D. = 2.875 in - 73.0 mm Wall = 0.203 in - 5.2 mm ASTM A500 Grade B or C Yield Strength = 60 ksi (min)	Comp = 44 kips - 196 kN Ten = 44 kips - 196 kN	ASTM A572 Grade 50 0.375 in - 9.5 std 0.50 in - 12.7 opt	(2) 0.75 in - 19.1 mm Grd 8 Bolts	none
Model TAF-288	O.D. = 2.875 in - 73.0 mm Wall = 0.262 in - 6.7 mm ASTM A500 Grade B or C Yield Strength = 60 ksi (min)	Comp = 80 kips - 356 kN Ten = 80 kips - 356 kN	ASTM A572 Grade 50 0.375 in - 9.5 std 0.50 in - 12.7 mm opt	(3) 0.75 in - 19.1 mm Grd 8 Bolts	ESR 3559
Model TAF-288H	O.D. = 2.875 in - 73.0 mm Wall = 0.262 in - 6.7 mm HRPOS LA Grade H490SI Yield Strength = 81 ksi (min)	Comp = 100 kips - 445 kN Ten = 100 kips - 445 kN	ASTM A572 Grade 50 0.375 in - 9.5 std 0.50 in - 12.7 mm opt	(3) 0.75 in - 19.1 mm Grd 8 Bolts	none
Model TAF-350	O.D. = 3.50 in - 88.9 mm Wall = 0.300 in - 7.6 mm ASTM A500 Grade B or C Yield Strength = 60 ksi (min)	Comp = 115 kips - 512 kN Ten = 120 kips - 512 kN	ASTM A572 Grade 50 0.375 in - 9.5 std 0.50 in - 12.7 mm opt	(3) 1.00 in - 25.4 mm Grd 8 Bolts	ESR 3559
Model TAF-450	O.D. = 4.50 in - 114.3 mm Wall = 0.337 in - 8.6 mm ASTM A500 Grade B or C Yield Strength = 50 ksi (min)	Comp = 160 kips - 712 kN Ten = 160 kips - 712 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm std 0.50 in - 12.7 mm opt	(3) 1.25 in - 28.6 mm Grd 8 Bolts	none
Model MTAH-55E	O.D. = 5.50 in - 139.7 mm Wall = 0.375 in - 9.53 mm	Comp = 240 kips - 1068 kN Ten = 240 kips - 1068 kN	ASTM A572 Grade 50 0.75 in - 19.05 mm std	(3) 1.25 in - 28.6 mm Grd 8 Bolts	none
Model TAF-663	O.D. = 6.625 in - 168.3 mm Wall = 0.280 in - 7.1 mm ASTM A500 Grade B or C Yield Strength = 50 ksi (min)	Comp = 200 kips - 890 kN Ten = 200 kips - 890 kN	ASTM A572 Grade 50 0.50 in - 12.7 mm opt 0.75 in - 18.04 mm opt	Weld or Bolt	none



ECP Helical Piles, Tiebacks & Anchors		Ultimate Capacity Based Upon Torque (kips - kN) (1) (2)	Helix Bearing Plate Grade & Thickness (in - mm)	Section Coupling Method	Building Code Certifications
Model MTAH-70E	O.D. = 6.625 in - 168.3 mm Wall = 0.432 in - 10.97 mm	Comp = 328 kips - 1459 kN Ten = 328 kips - 1459 kN	ASTM A572 Grade 50 0.75 in - 19.05 mm std	(3) 1.25 in - 28.6 mm Grd 8 Bolts	none
Model TAF-863	O.D. = 8.625 in - 219.1 mm Wall = 0.250 in - 6.4 mm ASTM A500 Grade B or C Yield Strength = 50 ksi (min)	Comp = 300 kips - 1335 kN Ten = 300 kips - 1335 kN	ASTM A572 Grade 50 0.50 in - 12.7 mm opt 0.75 in - 18.04 mm opt	Weld or Bolt	none

(1) The values shown only address torque correlated soil capacity. Other mechanical limit states of the pile/anchor, its couplers, and its connections to the structure (brackets) may also govern the design capacity. Refer to the manufacturer's technical manual for further information."

(2) Large diameter helical piles develop capacity by a combination of both end-bearing and skin friction. The ultimate pile capacity is calculated based on the site-specific soil profile on a case-by-case basis. Load tests are often recommended for larger shaft sizes to identify a site-specific torque correlation factor (Kt), to determine the pile displacement versus load, and to verify the helical pile configuration.