



IDEAL Group Helical Piles, Tiebacks & Anchors		Ultimate Capacity Based Upon Torque (1) (2)	Helix Bearing Plate Grade & Thickness (in - mm)	Section Coupling Method	Building Code Certifications
Round Corner Square Bar (RCS)					
Model SQ150	1.50 in - 38.1 mm ASTM A576, Grade 15V-30M Yield Strength = 90 ksi (min)	Comp = 70 kips - 311 kN Ten = 60 kips - 267 kN	ASTM A1018 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm	(1) 0.75 in - 19.05 mm ASTM A325 Bolt	ICC ESR-3750
Model SQ175	1.75 in - 44.5 mm ASTM A576 Grade 15V-30M Yield Strength = 90 ksi (min)	Comp = 110 kips - 489 kN Ten = 95 kips - 423 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm	(1) 0.875 in - 22.23 mm Grade B7	None
Model SQ200	2.00 in - 50.8 mm ASTM A576 Grade 15V-30M Yield Strength = 90 ksi (min)	Comp = 150 kips - 667 kN Ten = 130 kips - 579 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm	(1) 1.125 in - 25.40 mm Grade B7	None
Round Shaft					
Model 238190	O.D. = 2.375 in - 60.3 mm Wall = .190 in - 4.8 mm Yield Strength = 80 ksi (min)	Comp = 50 kips - 222 kN Ten = 50 kips - 222 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm	(2) 0.75 in - 19.1 mm ASTM A325 Bolts	None
Model 278203	O.D. = 2.875 in - 73.0 mm Wall = 0.203 in - 5.17 mm ASTM A500 Yield Strength = 80 ksi (min)	Comp = 72 kips - 320 kN Ten = 56 kips - 250 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm	(2) 0.75 in - 19.1 mm ASTM A325 Bolts	ICC ESR-3750
Model 278276	O.D. = 2.875 in - 73.0 mm Wall = 0.276 in - 7.0 mm ASTM A500 Yield Strength = 80 ksi (min)	Comp = 90 kips - 400 kN Ten = 70 kips - 311 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm	(2) 0.75 in - 19.1 mm ASTM A325 Bolts	ICC ESR-3750
Model 312216	O.D. = 3.50 in - 88.9 mm Wall = 0.216 in - 5.5 mm ASTM A500 Yield Strength = 80 ksi (min)	Comp = 91 kips - 405 kN Ten = 85 kips - 378 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm	(2) 0.75 in - 19.1 mm ASTM A325 Bolts	ICC ESR-3750
Model 312300	O.D. = 3.50 in - 88.9 mm Wall = 0.30 in - 7.62 mm ASTM A500 Yield Strength = 80 ksi (min)	Comp = 120 kips - 534 kN Ten = 102 kips - 454 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm 0.750 in - 19.1 mm	(2) 0.75 in - 19.1 mm ASTM A325 Bolts	ICC ESR-3750
Model 412337	O.D. = 4.50 in - 114.3 mm Wall = 0.337 in - 8.6 mm Yield Strength = 80 ksi (min)	Comp = 148 kips - 658 kN Ten = 132 kips - 587 kN	ASTM A572 Grade 50 0.375 in - 9.5 mm 0.500 in - 12.7 mm 0.625 in - 15.9 mm 0.750 in - 19.1 mm	(2) 1.00 in - 25.4 mm ASTM A325 bolts	None
Model 512361	O.D. = 5.50 in - 139.7 mm Wall = 0.361 in - 9.14 mm Yield Strength = 80 ksi (min)	(2)	ASTM A572 Grade 50 0.500 in - 12.7 mm 0.625 in - 15.9 mm 0.750 in - 19.1 mm 0.875 in - 22.2 mm 1.000 in - 25.4 mm	Project Specific	Not applicable
Model 512415	O.D. = 5.50 in - 139.7 mm Wall = 0.415 in - 10.54 mm Yield Strength = 80 ksi (min)	(2)	ASTM A572 Grade 50 0.500 in - 12.7 mm 0.625 in - 15.9 mm 0.750 in - 19.1 mm 0.875 in - 22.2 mm 1.000 in - 25.4 mm	Project Specific	Not applicable
Model 700408	O.D. = 7.00 in - 177.8 mm Wall = 0.408 in - 10.4 mm Yield Strength = 80 ksi (min)	(2)	ASTM A572 Grade 50 0.500 in - 12.7 mm 0.625 in - 15.9 mm 0.750 in - 19.1 mm 0.875 in - 22.2 mm 1.000 in - 25.4 mm	Project Specific	Not applicable

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Model 700453	O.D. = 7.00 in - 177.8 mm Wall = 0.453 in - 11.5 mm Yield Strength = 80 ksi (min)	(2)	ASTM A572 Grade 50 0.500 in - 12.7 mm 0.625 in - 15.9 mm 0.750 in - 19.1 mm 0.875 in - 22.2 mm 1.000 in - 25.4 mm	Project Specific	Not applicable
Model 858322	O.D. = 8.625 in - 219.1 mm Wall = 0.322 in - 8.2 mm Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 0.625 in - 15.9 mm 0.750 in - 19.1 mm 0.875 in - 22.2 mm 1.000 in - 25.4 mm 1.250 in - 31.8 mm	Project Specific	Not applicable
Model 1034	O.D. = 10.75 in - 273.1 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 0.750 in - 19.1 mm 0.875 in - 22.2 mm 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm	Project Specific	Not applicable
Model 1234	O.D. = 12.75 in - 323.9 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 0.750 in - 19.1 mm 0.875 in - 22.2 mm 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm	Project Specific	Not applicable
Model 1600	O.D. = 16.00 in - 406.4 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 0.875 in - 22.2 mm 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm 1.750 in - 44.5 mm	Project Specific	Not applicable
Model 1800	O.D. = 18.00 in - 457.2 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm 1.750 in - 44.5 mm	Project Specific	Not applicable
Model 2000	O.D. = 20.00 in - 508.0 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm 1.750 in - 44.5 mm	Project Specific	Not applicable
Model 2400	O.D. = 24.00 in - 609.6 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm 1.750 in - 44.5 mm 2.000 in - 50.8 mm	Project Specific	Not applicable
Model 3000	O.D. = 30.00 in - 762.0 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm 1.750 in - 44.5 mm 2.000 in - 50.8 mm	Project Specific	Not applicable
Model 3600	O.D. = 36.00 in - 914.4 mm Wall = Dependent on Availability Yield Strength = 50 ksi (min)	(2)	ASTM A572 Grade 50 1.000 in - 25.4 mm 1.250 in - 31.8 mm 1.500 in - 38.1 mm 1.750 in - 44.5 mm 2.000 in - 50.8 mm	Project Specific	Not applicable

(1) The values shown only address torque correlated soil capacity. Other mechanical limit states of the pile/anchor, its couplers, and its connections to the structure (brackets) may also govern the design capacity. Refer to the manufacturer's technical manual for further information.

(2) Large diameter helical piles develop capacity by a combination of both end-bearing and skin friction. The ultimate pile capacity is calculated based on the site-specific soil profile on a case-by-case basis. Load tests are often recommended for larger shaft sizes to identify a site-specific torque correlation factor (Kt), to determine the pile displacement versus